DRAWING INDEX

GROUND FLOOR PLAN A2 1st & 2nd FLOOR PLANS EXTERIOR BUILDING ELEVATIONS A3 A4 EXTERIOR BUILDING ELEVATIONS A5 ROOF PLAN A6 CROSS SECTION & CONSTRUCTION **DETAILS** SCHEMATIC PILING PLAN ELECTRICAL PLANS E1GENERAL NOTES & SPECIFICATIONS SP

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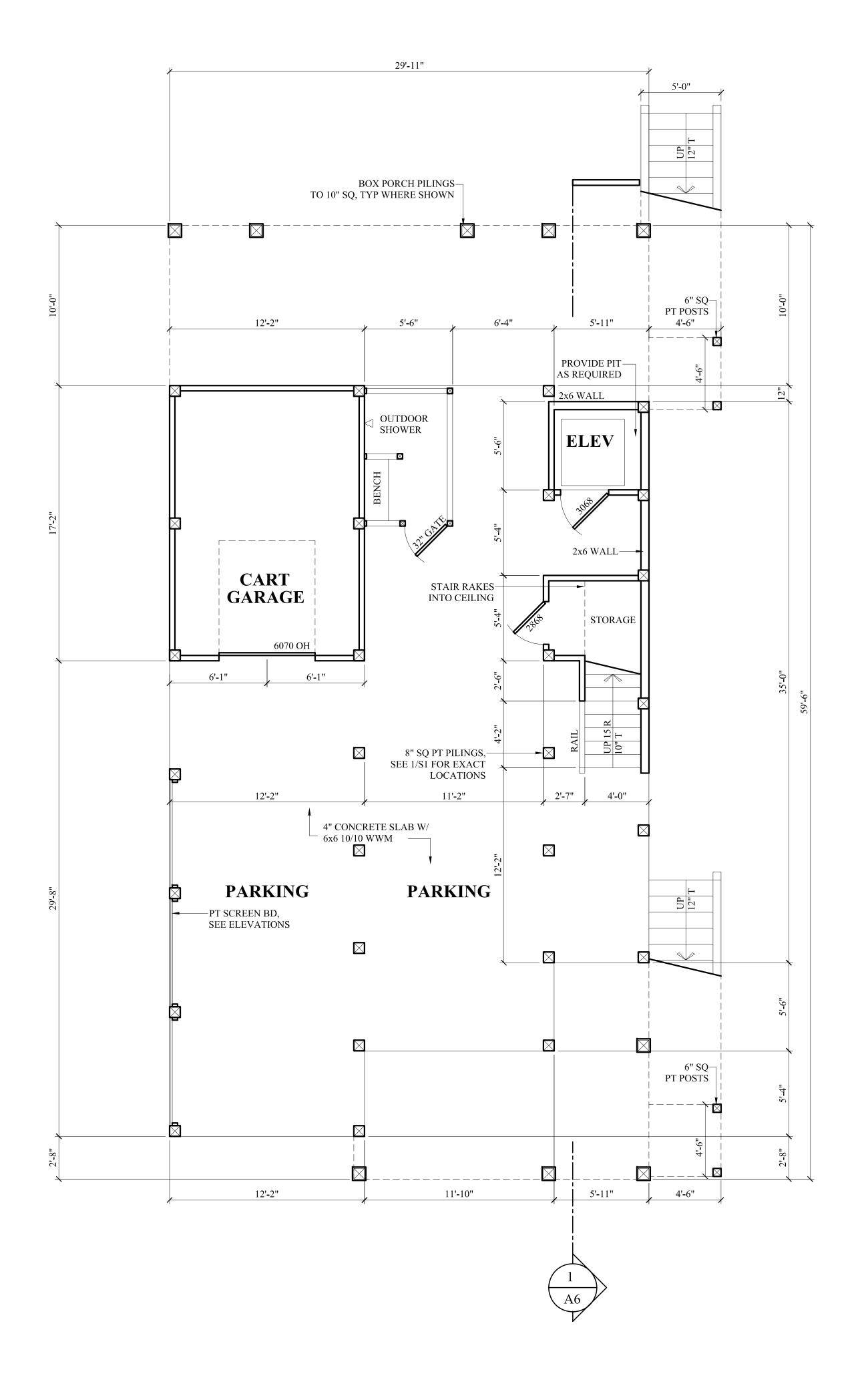
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AREA CALCULATIONS

FIRST FLOOR HEATED SECOND FLOOR HEATED	1268 S 1203 S
TOTAL HEATED AREA	2471 S
GARAGE COVERED PORCH	209 S 656 S 67 S

ABBREVIATIONS

AFF	ABOVE FINISHED FLOOR
AHU	AIR HANDLING UNIT
CATH	CATHEDRAL
CLG	CEILING
CONT	CONTINUOUS
DIA	DIAMETER
DBL	DOUBLE
DO	DOUBLE OVEN
DW	DISHWASHER
ELEV	ELEVATOR
FB	FLUSH BEAM, sized by others
FR	FRENCH
GWB	GYPSUM WALL BOARD
HT	HEIGHT
JST	JOIST
MO	MASONRY OPENING
O/C	ON CENTER
PT	PRESSURE TREATED
PDS	PULL-DOWN STAIR
QTR	QUARTER
R	RISER
REF	REFRIGERATOR
RO	ROUGH OPENING
SQ	SQUARE
SHWR	SHOWER
T	TREAD
T&G	TONGUE AND GROOVE
UNO	UNLESS NOTED OTHERWISE
W/	WITH
WWM	WELDED WIRE MESH

GROUND FLOOR PLAN

3) DOOR & WINDOW SIZE CLARIFICATION EXAMPLE:

4) CONTRACTOR TO VERIFY ELEVATOR SHAFT OPENINGS

NOTE: IT IS RECOMMENDED THESE STRUCTURAL PLANS BE

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1) 8'-0" CEILING HT

2) DIMENSIONS ARE TO FRAMING

PRIOR TO CONSTRUCTION

CONSTRUCTION & PERMITTING

2468 = 2'-4" x 6'-8" (NOMINAL SIZE)

ABOVE FINISHED FLOOR AIR HANDLING UNIT	Sullivan Design Co. does not assume any liability for the construction methods or any deviation from these plar All information on these plans is to be verified prior to the commencement of construction. Should any discrepancies contact the Sullivan Design Co. prior to any deviation from this plan. For construction purposes, do not scale the plans. Refer to dimensions indicated on these plans.
CATHEDRAL	ility m d scr scr bl
CEILING	ab fro ifie di do do
CONTINUOUS	in ver ver any any the the
DIAMETER	any atrio d c
OOUBLE	e e e e e e e e e e e e e e e e e e e
OOUBLE OVEN	um de to Sho Sho srio pur
DISHWASHER	assume any devi s is to b n. Shoul o. prior on purpe dicated e
ELEVATOR	t anns nns con Coortion in d
FLUSH BEAM, sized by others	not or o plans uctio yn Co yn Co ructio
FRENCH	es de str str ssic nst ion
GYPSUM WALL BOARD	does ethods hese constr cons ensior drawii
HEIGHT	Co. on the of color livan For dime
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MASONRY OPENING	gn tio Su Su an.
ON CENTER	esi ruc atid em er ple er
PRESSURE TREATED	nst rm nce nce the
PULL-DOWN STAIR	Sullivan Design the construction All information commencement contact the Sul from this plan. plans. Refer to
QUARTER	ulliy briting om om ans
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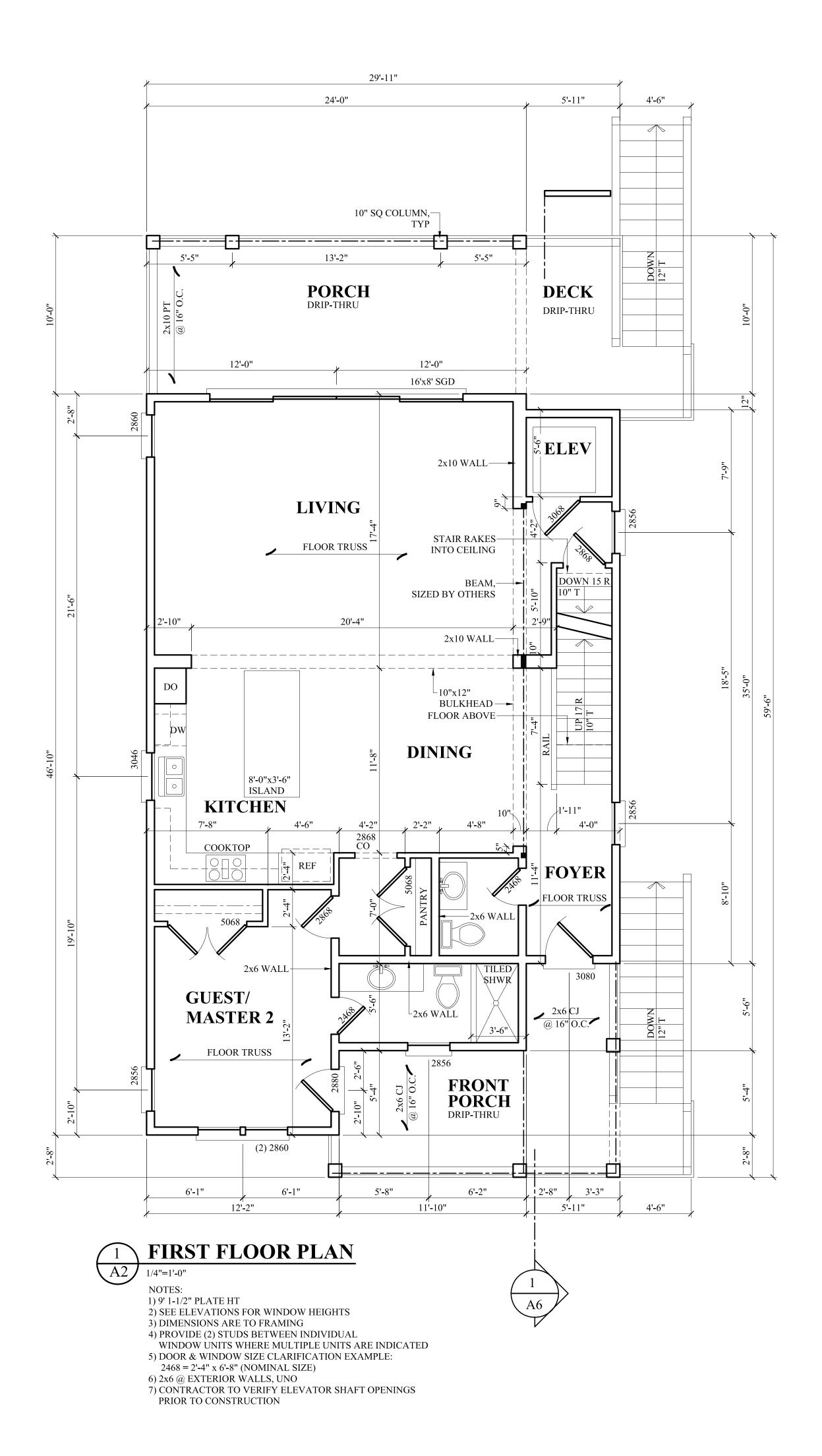
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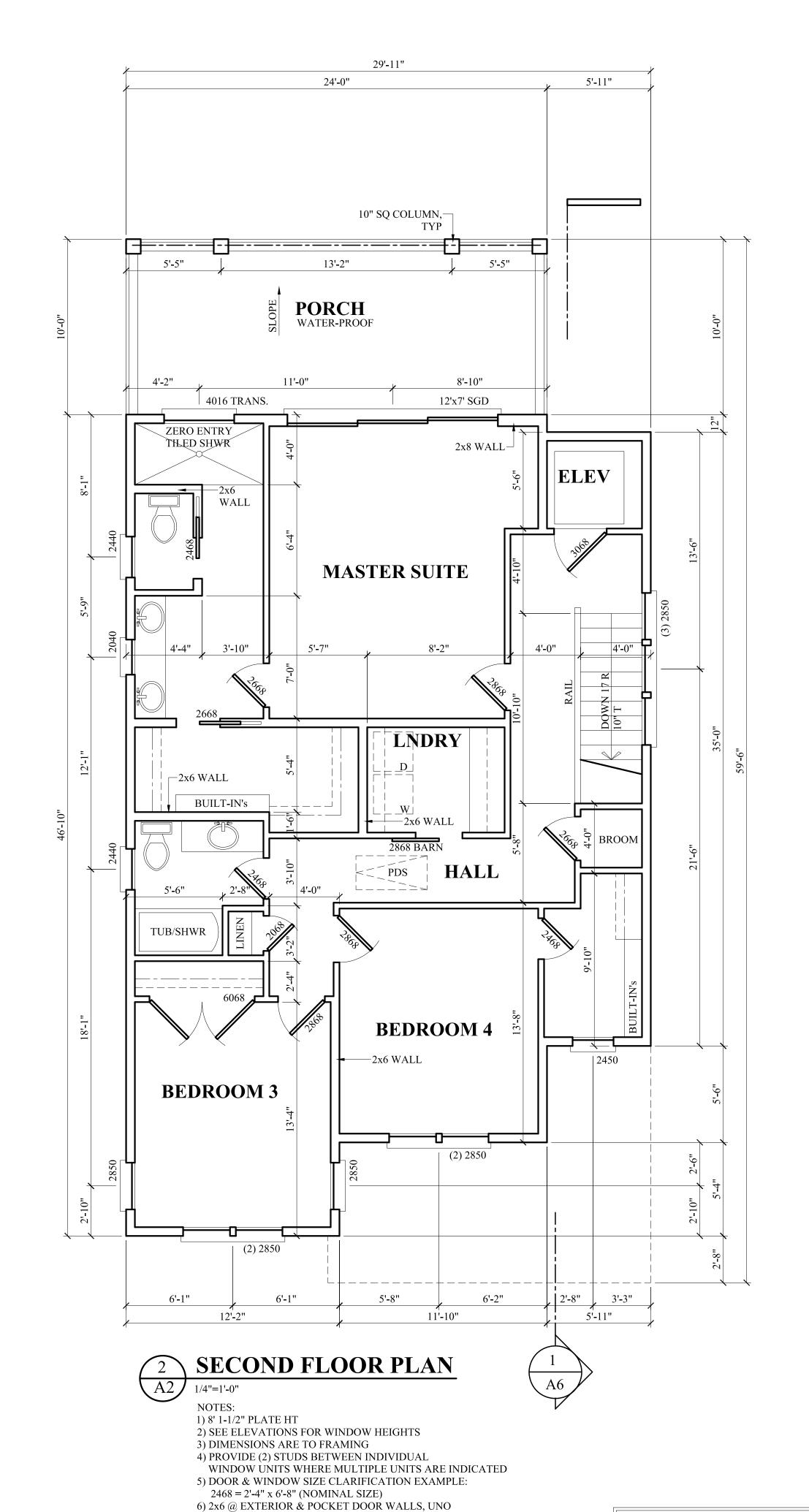
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7) CONTRACTOR TO VERIFY ELEVATOR SHAFT OPENINGS

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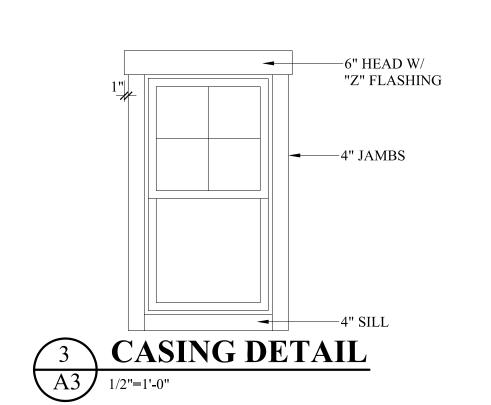
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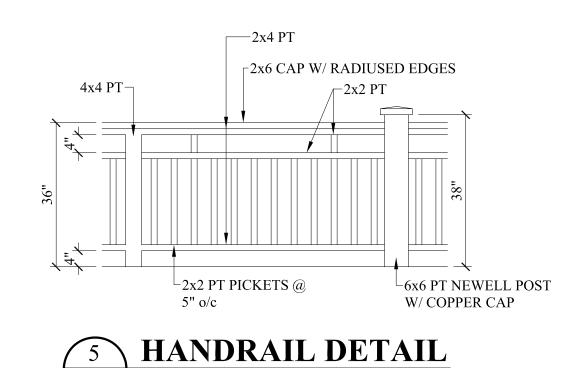
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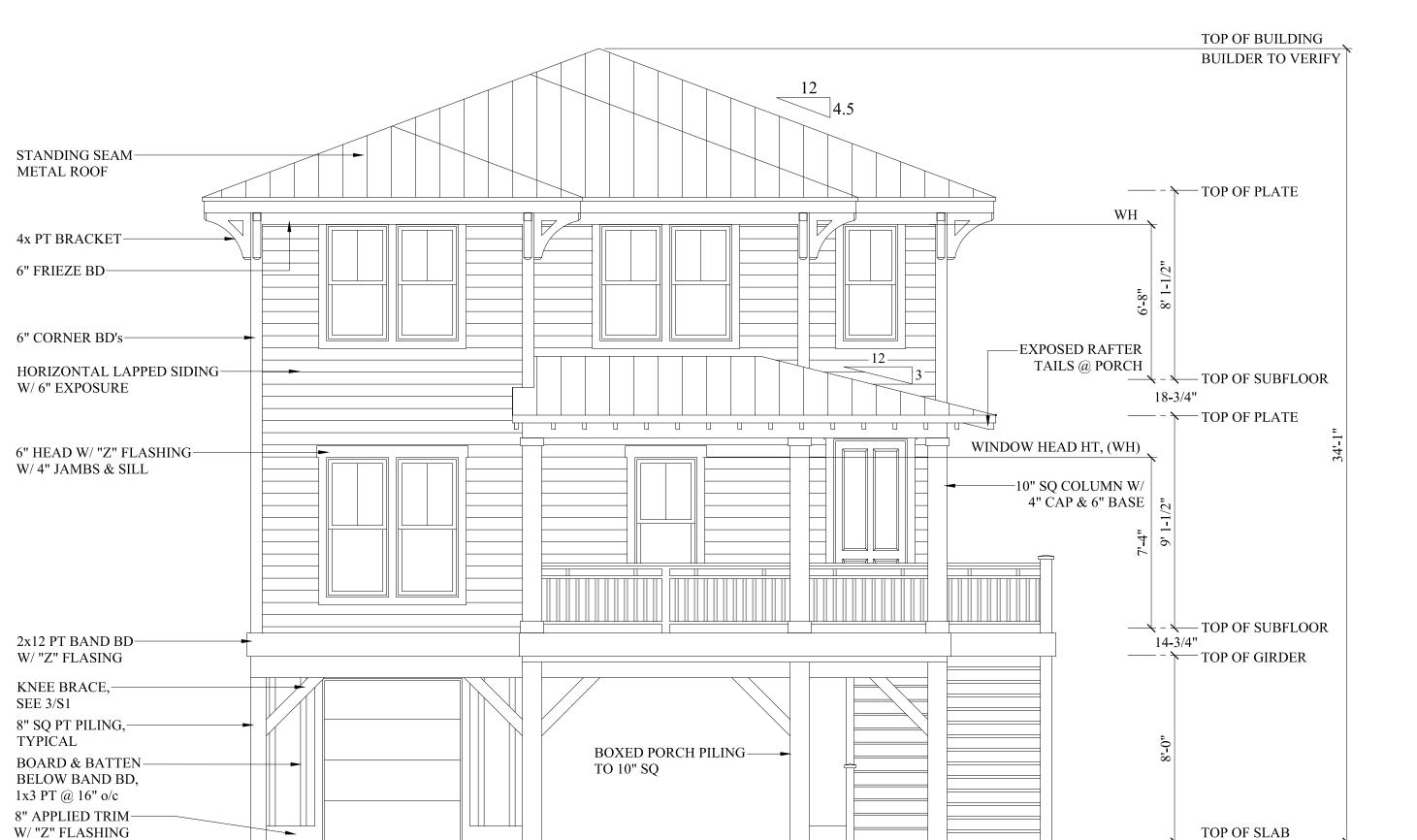


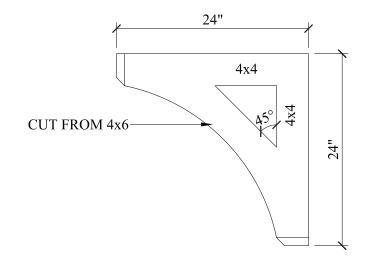
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RIGHT SIDE ELEVATION A3 1/4"=1'-0"

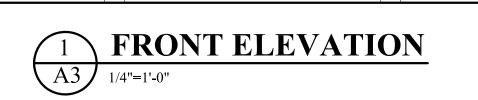












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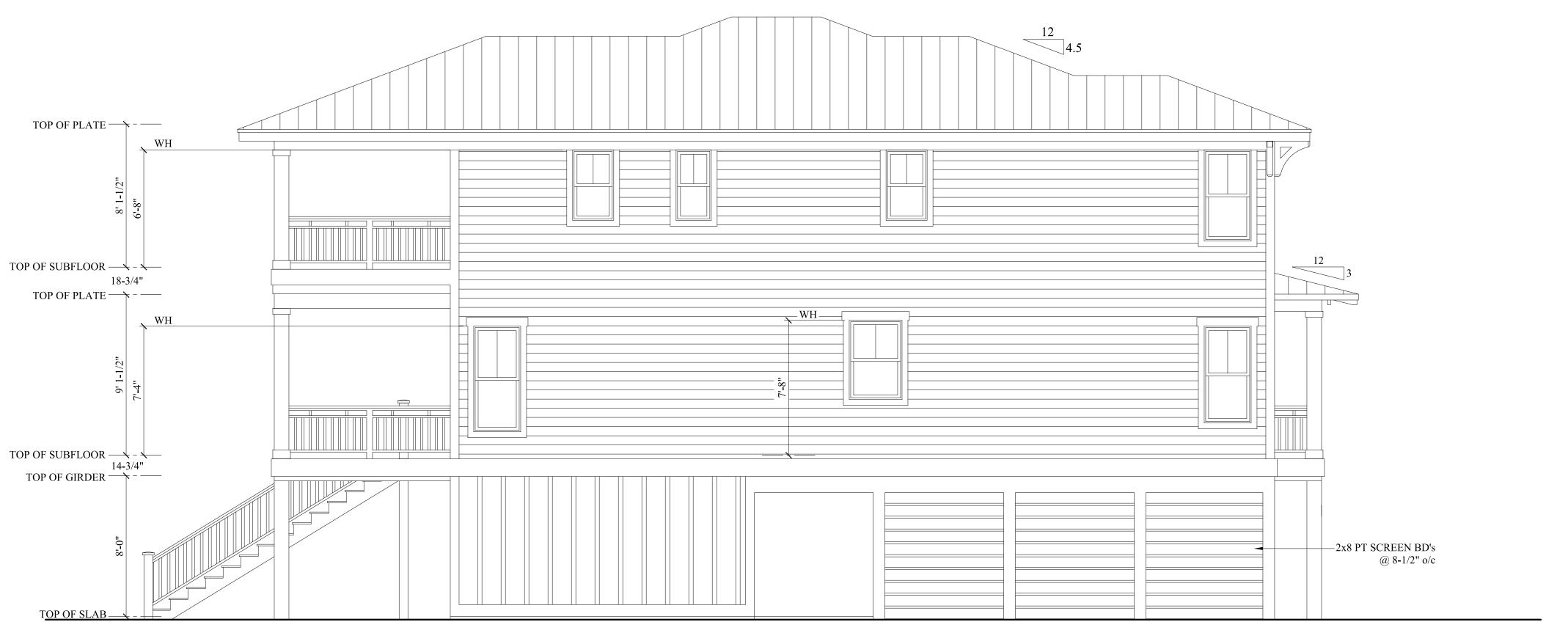
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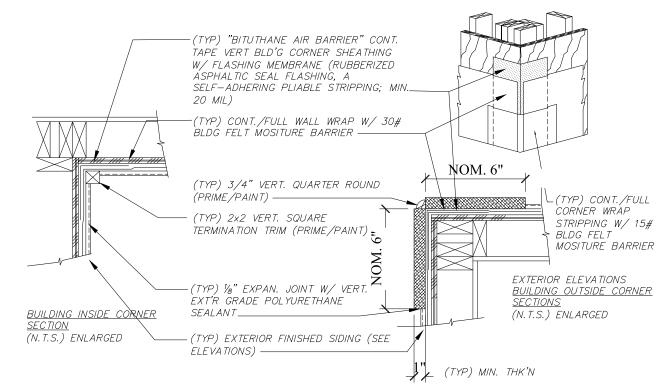
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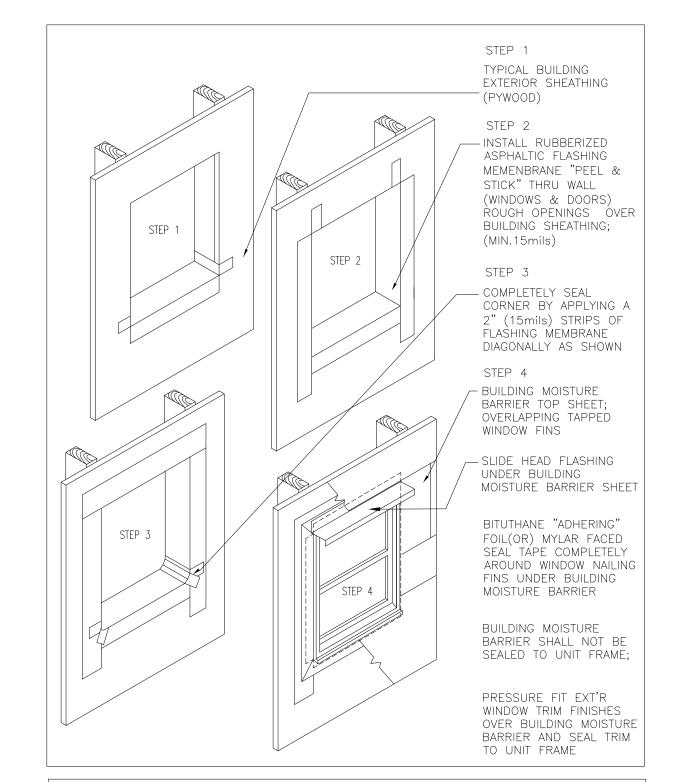


3 TYPICAL EXTERIOR CORNERS
NTS

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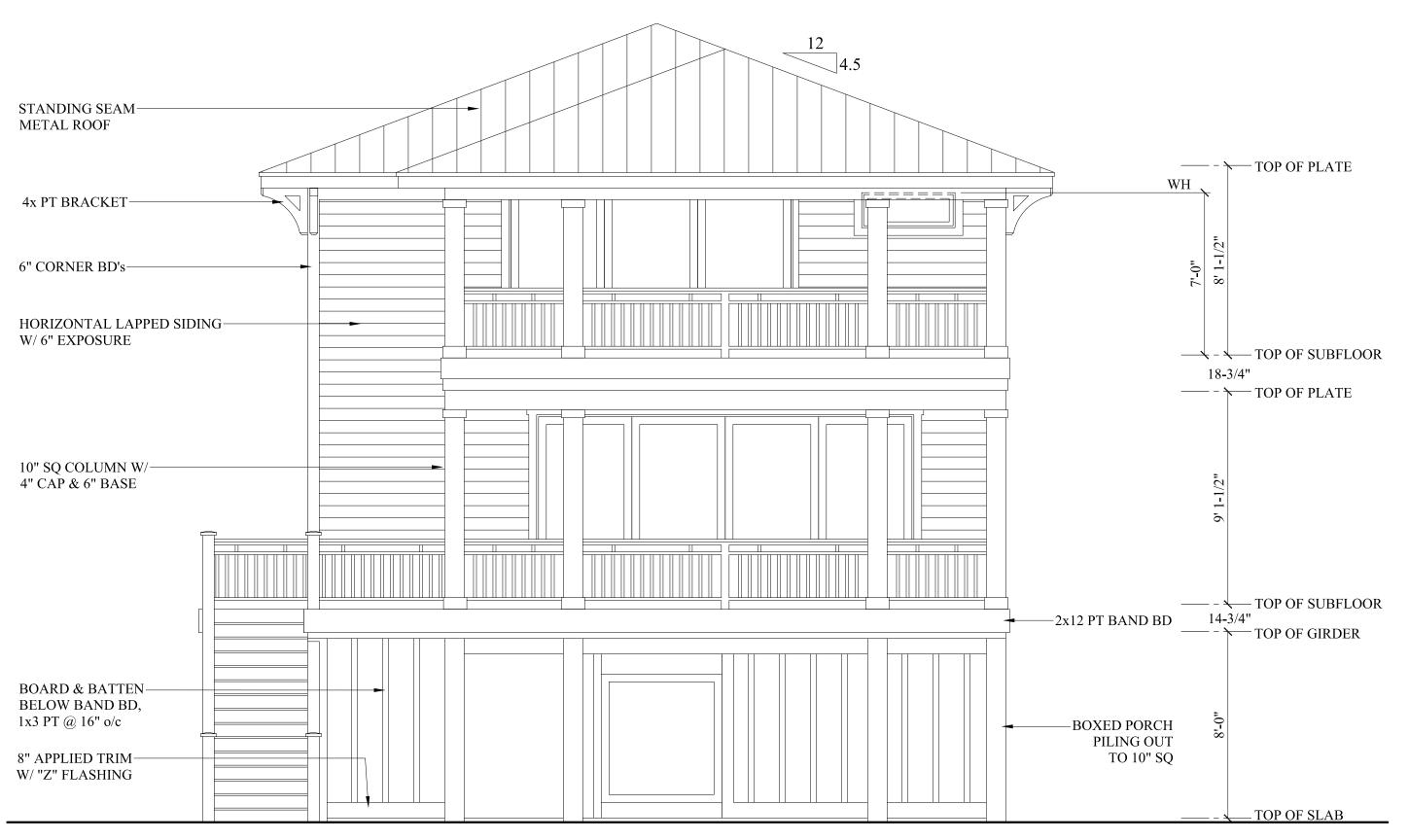
NOTE: SEE SHEET "A3" FOR TYPICAL MATERIAL INFO

2 LEFT SIDE ELEVATION
A4 1/4"=1'-0"



EXTERIOR WRAP AROUND PERIMETER BUILDING SHEATHING OF WINDOWS & DOORS (EXTENDING THRU ROUGH OPENING ALL FOUR EDGES) W/ MYLAR FACE FINISHED RUBBERIZED ASPHALTIC FLASHING; A SELF—ADHERING PLIABLE STRIP MEMBRANE (MIN. 15mils); MIN. 6" VERT. & HORZ SURFACE MITER CORNERS COVERAGE) (SIMILAR PRODUCT FLASHING MFG: W.R.GRACE "ICE AND WATER SHIELD)

4 TYP WINDOW FLASHING DTL
A4 NTS



REAR ELEVATION

A4 1/4"=1'-0"

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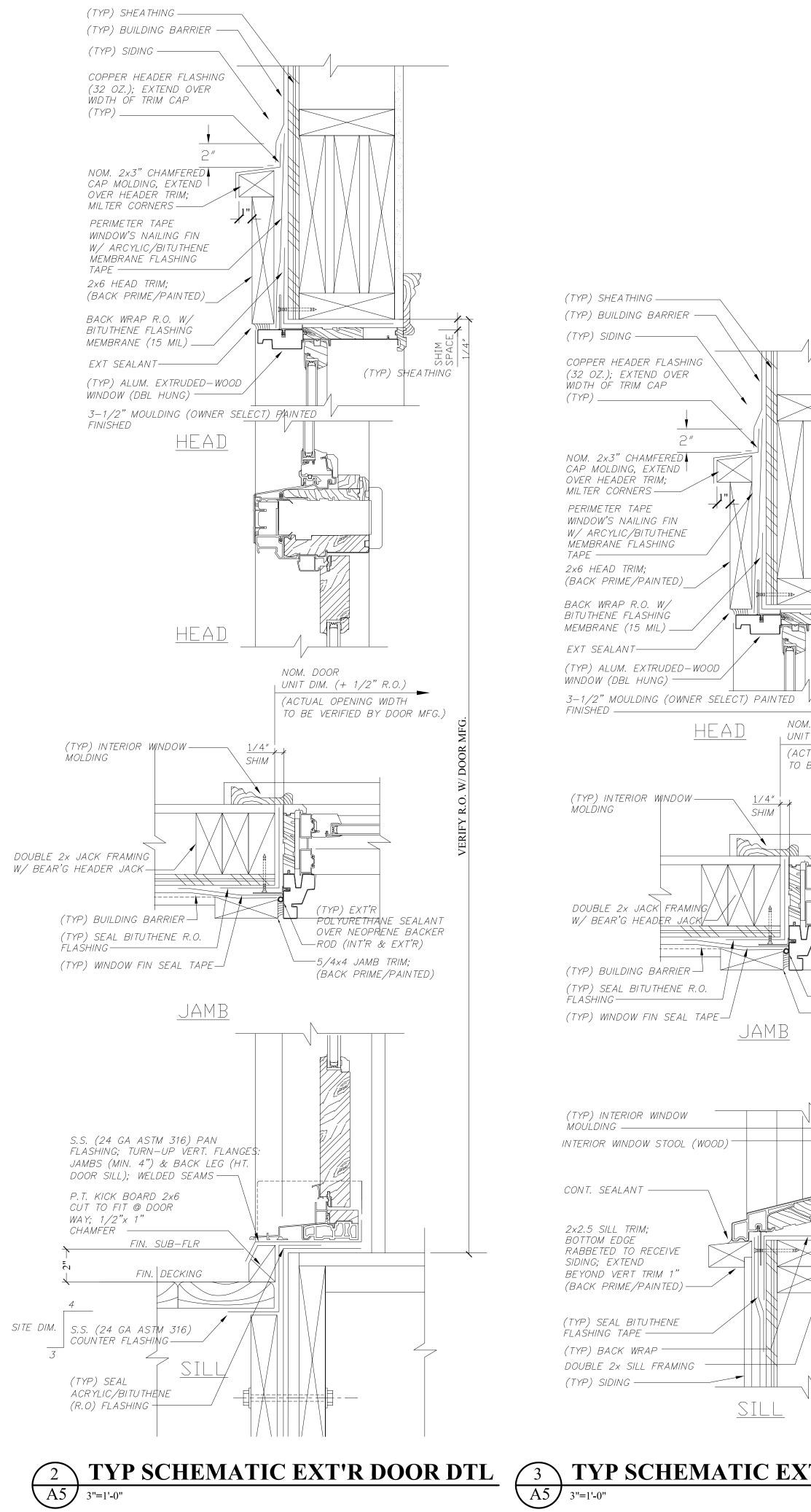
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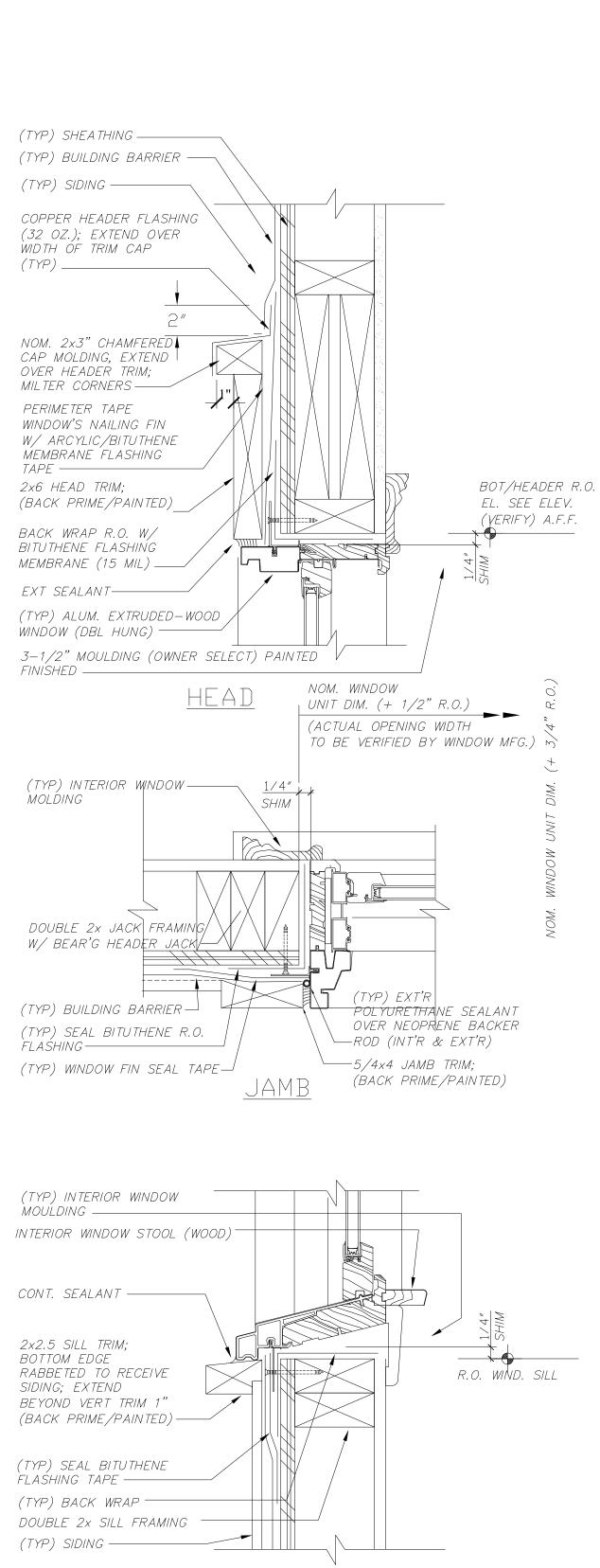
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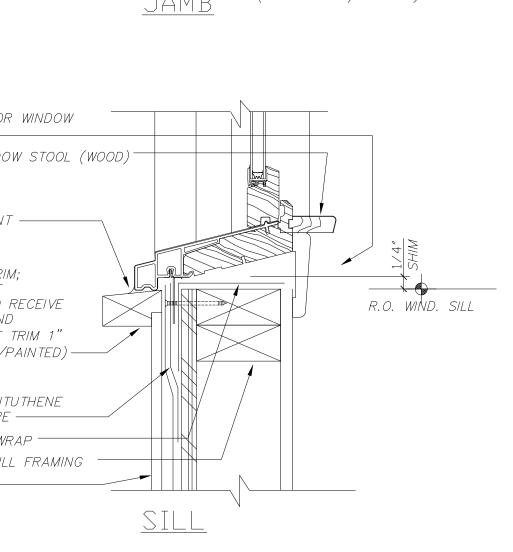
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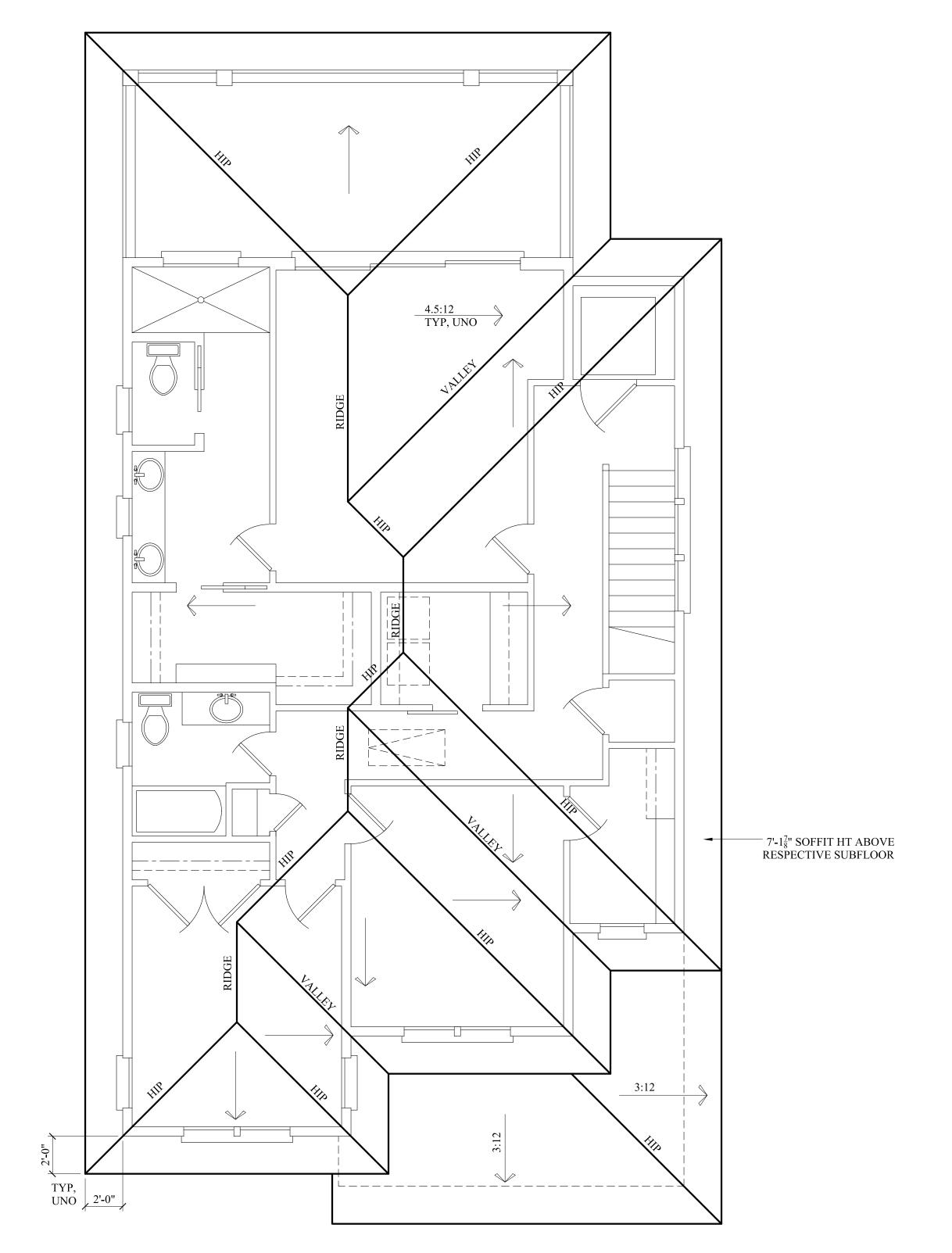
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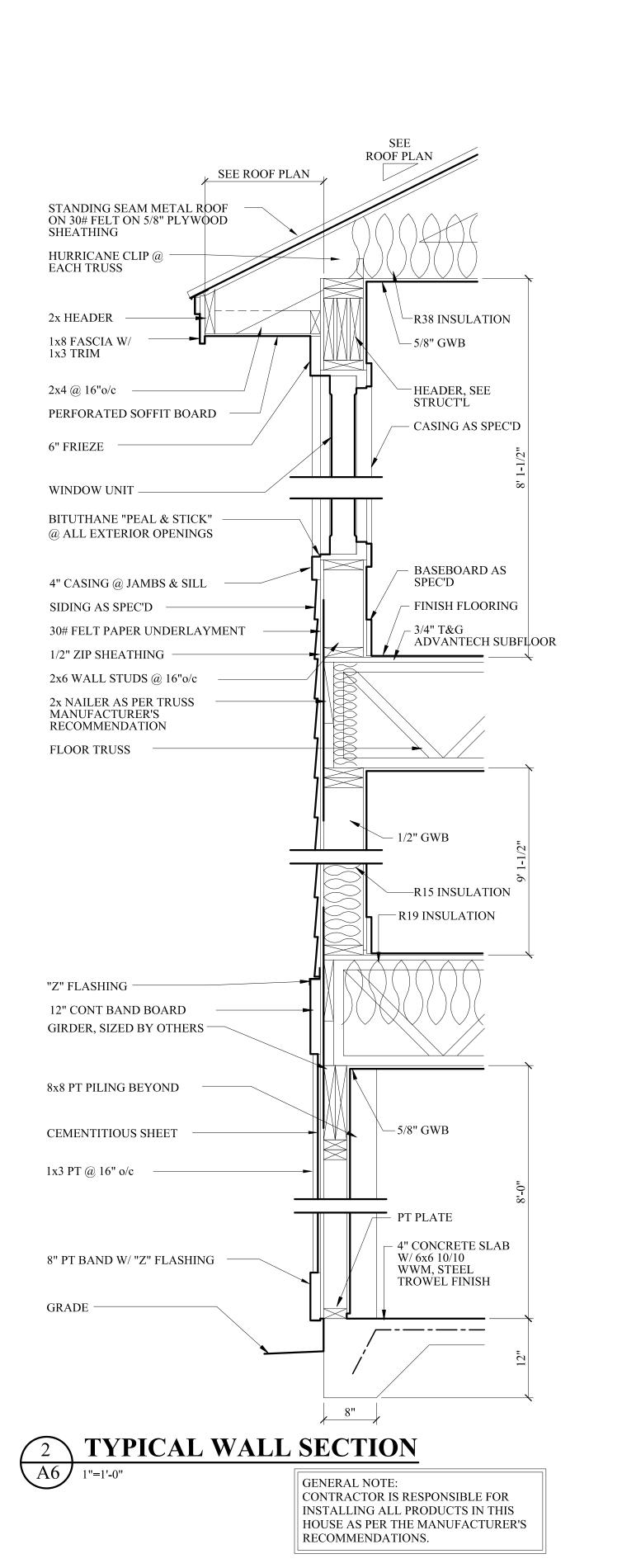
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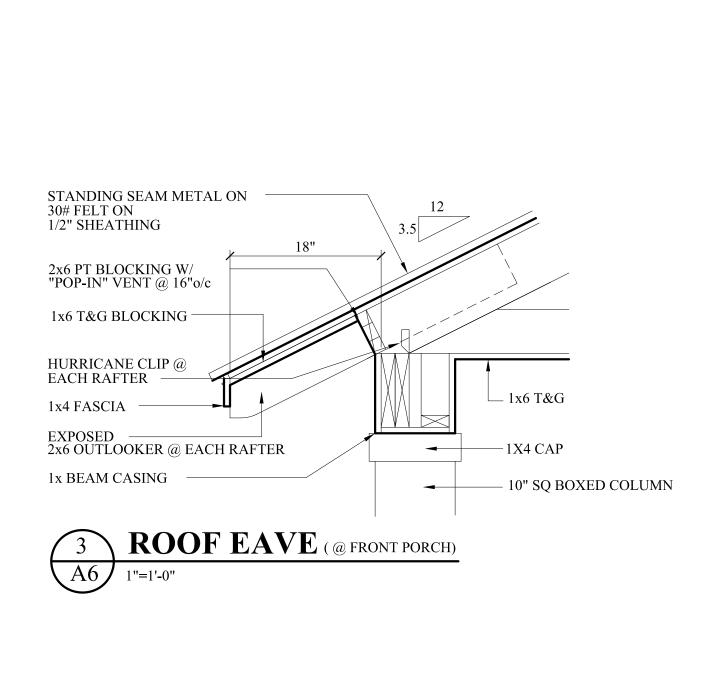
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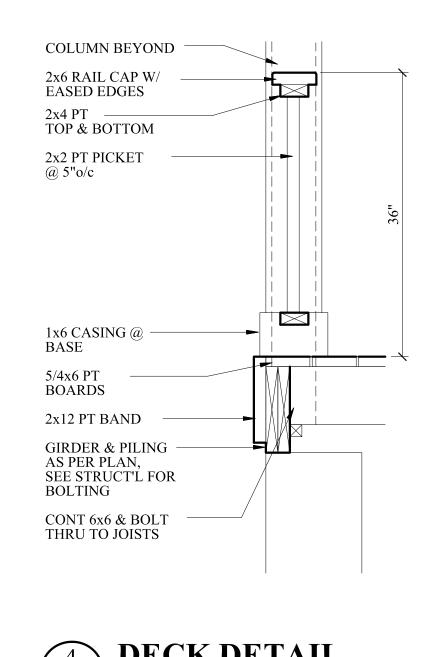
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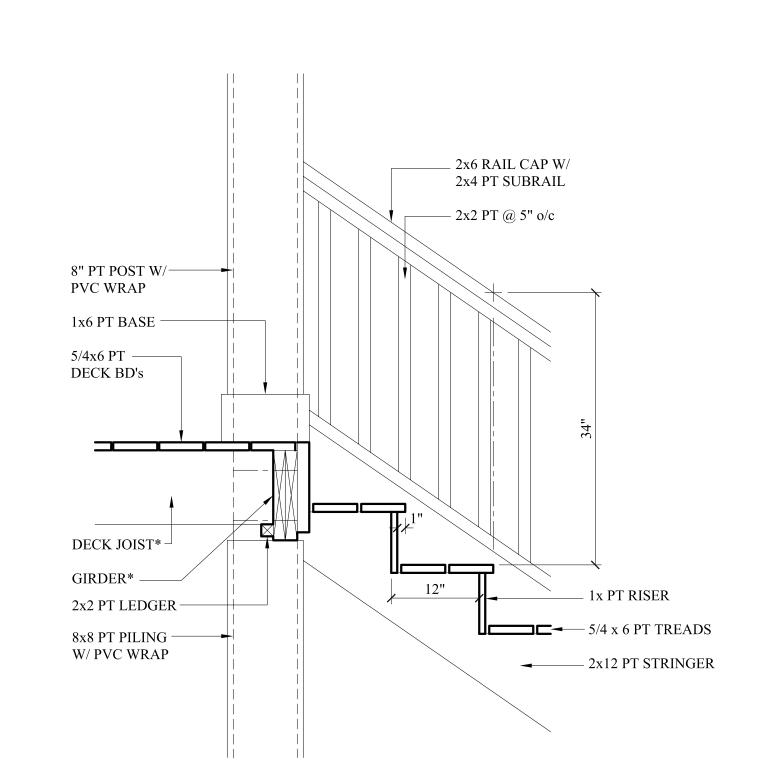
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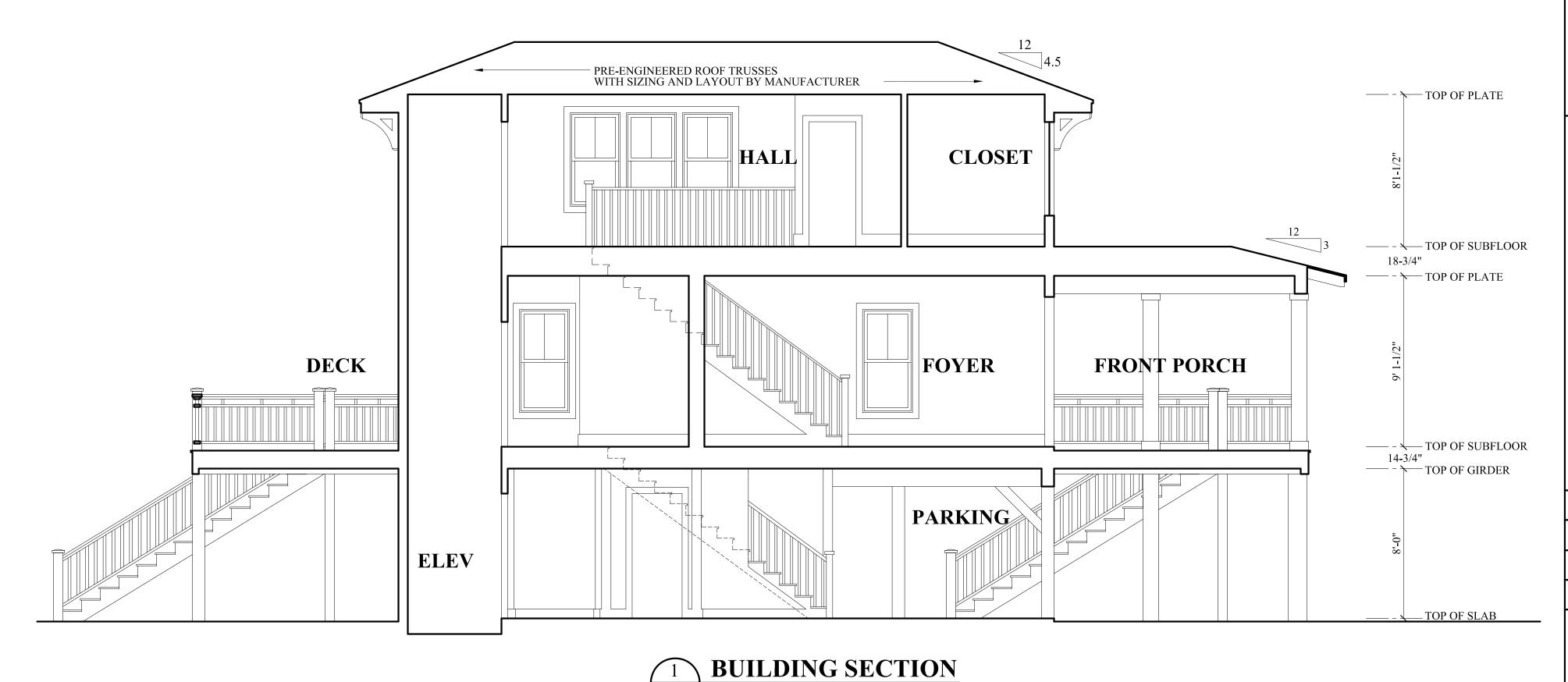


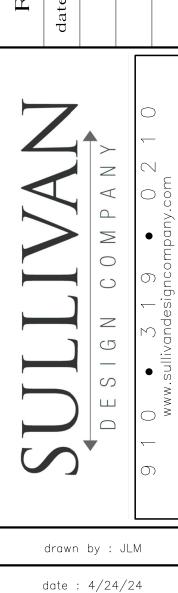












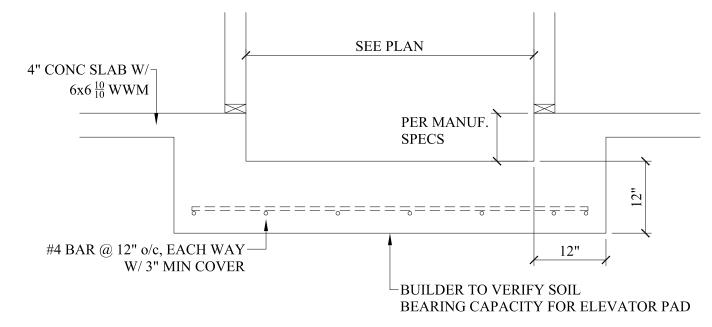
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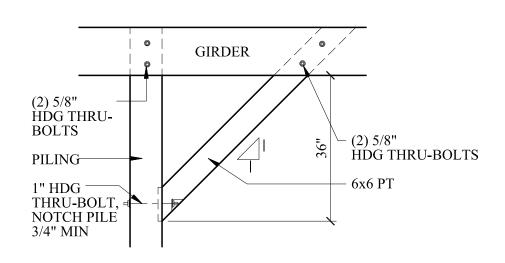
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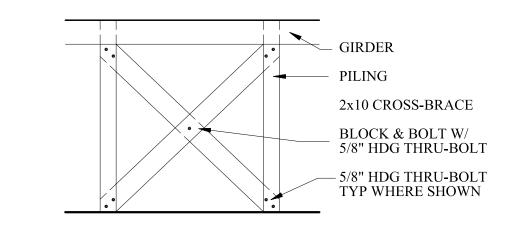
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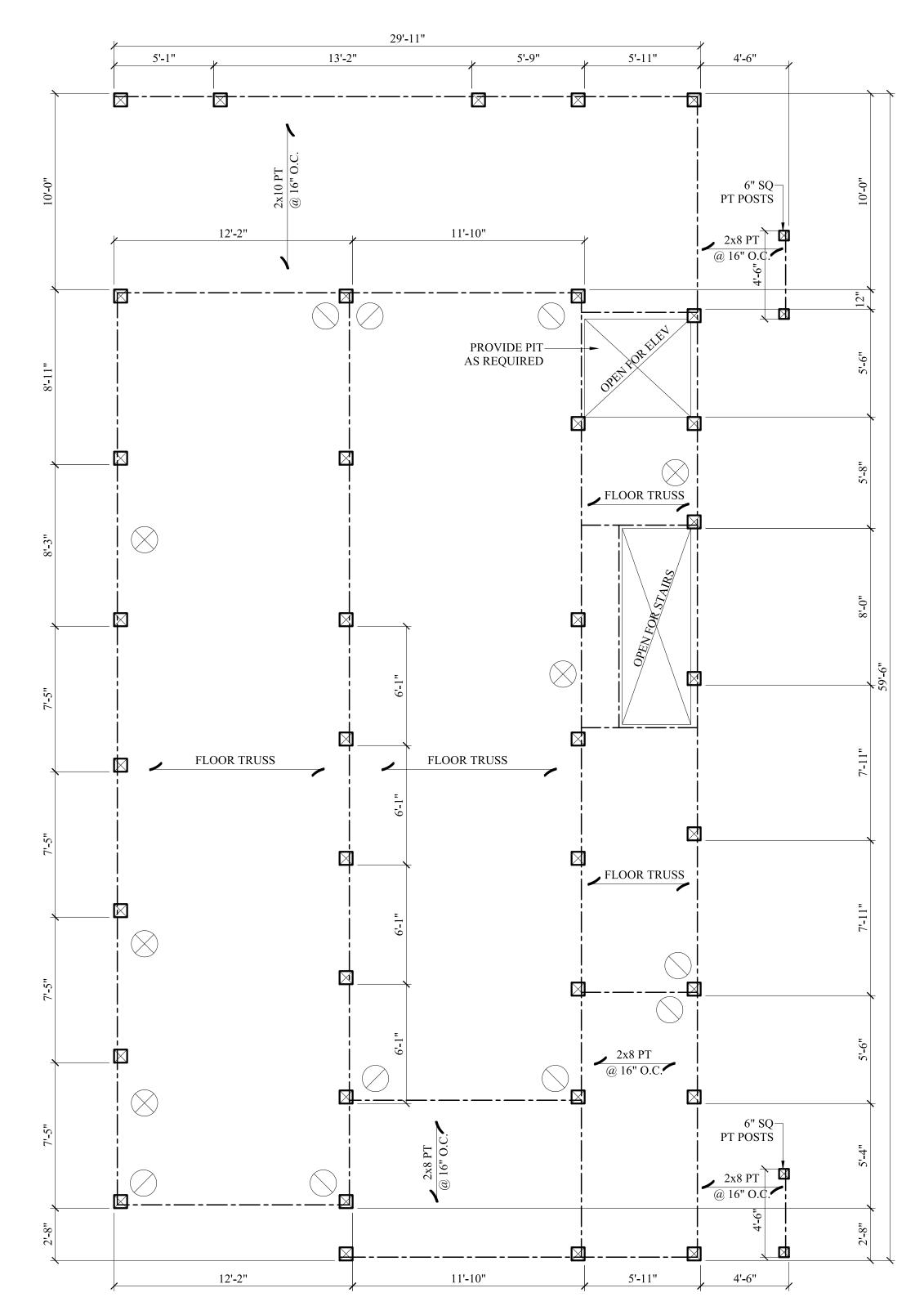
4 ELEVATOR PIT S1 NO SCALE











SCHEMATIC PILING/ 1 1st FLOOR FRAMING PLAN S1 1/4"=1'-0"

CROSS BRACE, SEE 2/S1

KNEE BRACE, SEE 3/S1

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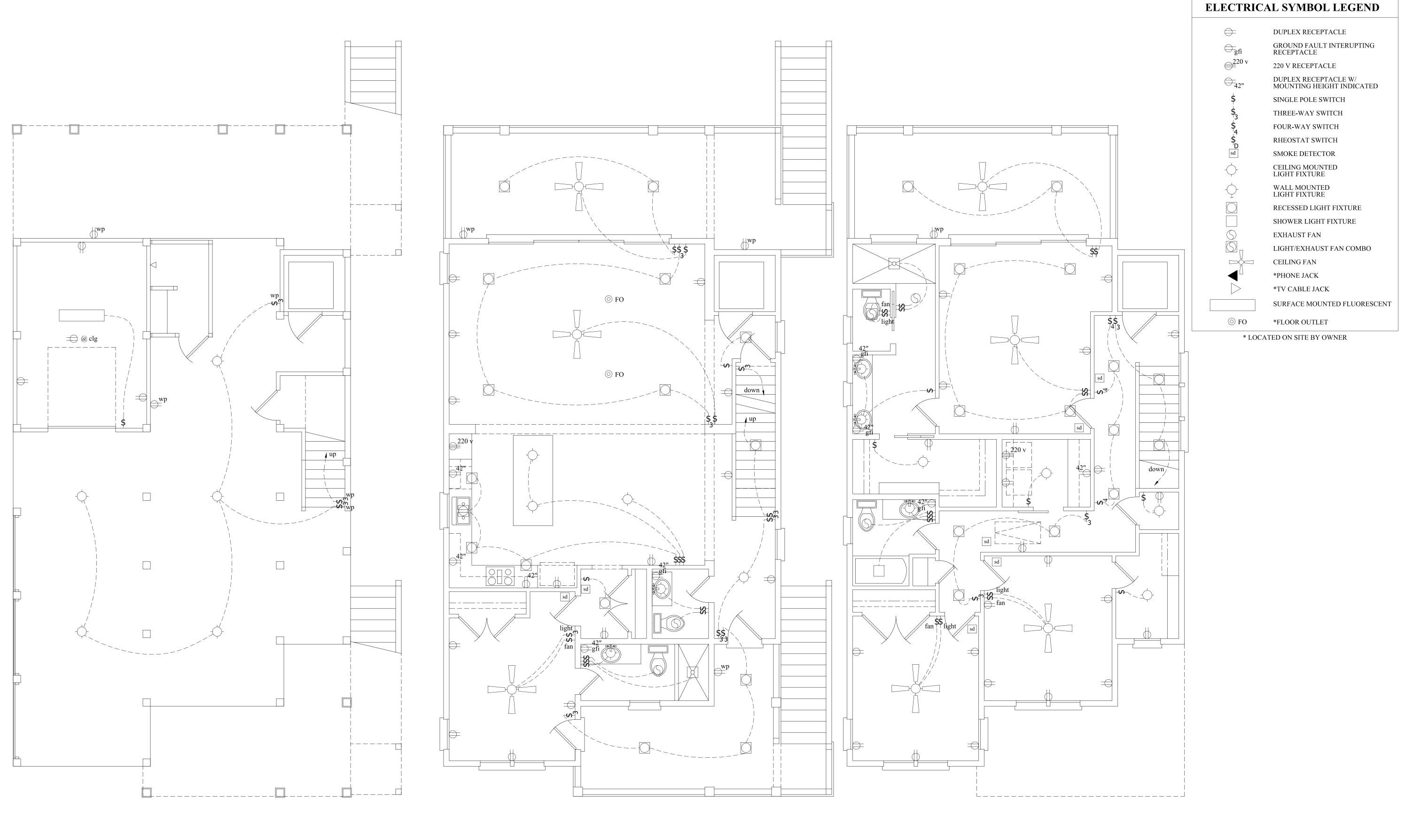
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GROUND FLOOR ELECTRICAL PLAN
E1 1/4"=1'-0"

2 1st FLOOR ELECTRICAL PLAN
E1 1/4"=1'-0"

3 2nd FLOOR ELECTRICAL PLAN
E1 1/4"=1'-0"

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	1.1	100111105	3000 1 51
	1.2	SLABS-ON-GRADE	4000 PSI
	1.3	ELEVATED SLABS	3500 PSI
•		· · · · · · · · · · · · · · · · · · ·	ACED IN ACCORDANCE WITH ACI 318 LATEST EDITION "BUILDING CODE REQUIREMENTS ITION "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDING"

LIGHTWEIGHT CONCRETE SHALL HAVE A WEIGHT OF 115 PCF. (CONCRETE NOT SPECIFICALLY NOTED AS LIGHTWEIGHT SHALL BE NORMAL

AS PER R301.4

AS PER R301.4

AS PER R301.4

40 PSF

30 PSF 40 PSF

40 PSF

AS PER TABLE R301.6

GROUT FOR BASE PLATES SHALL BE NON-SHRINKAGE GROUT AND HAVE A MINIMUM COMPRESSIVE STRENGTH (Fc) AT 28 DAYS OF 5000 PSI.

THE CONCRETE SLAB ON GRADE HAS BEEN DESIGNED USING A SUBGRADE MODULUS OF k= 250 PCI AND A DESIGN LOADING OF 200 PSF. THE SER IS NOT RESPONSIBLE FOR DIFFERENTIAL SETTLEMENT, SLAB CRACKING OR OTHER FUTURE DEFECTS RESULTING FROM UNREPORTED CONDITIONS MITIGATING THE ABOVE ASSUMPTIONS

CONTROL JOINTS SHALL BE SPACED IN SLABS ON GRADE AT A MAXIMUM OF 20'-0" O.C. UNLESS NOTED OTHERWISE.

CONTROL JOINTS SHALL BE PRODUCED USING CONVENTIONAL PROCESSES WITHIN 4 TO 12 HOURS AFTER THE SLAB HAS BEEN FINISHED.

REINFORCING STEEL SHALL NOT EXTEND THROUGH THE CONTROL JOINT.

ALL WELDED WIRE FABRIC FOR CONCRETE SLAB ON GRADE SHALL BE SUPPLIED IN FLAT SHEETS. THE WELDED WIRE FABRIC SHALL BE PLACED 2" FROM THE TOP OF SLAB AND SECURELY SUPPORTED DURRING THE CONCRETE POUR.

REINFORCING STEEL

REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60.

DETAILING, FABRICATION, AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 315 LATEST EDITION "MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES. HORIZONTAL FOOTING AND WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90° BENDS OR CORNER BARS SHALL BE

INSTALLED. THE CORNER BAR SHALL HAVE THE SAME SIZE AND SPACING AS THE HORIZONTAL REINFORCEMENT WITH A CLASS B TENSION LAP REINFORCEMENT AS REQUIRED A MINIMUM OF 40 BAR DIAMETERS FOR TENSION OR COMPRESSION UNLESS NOTED OTHERWISE. SPLICES

WHERE REINFORCING DOWELS ARE REQUIRED THEY SHALL BE EQUILIVAENT SIZE AND SPACING AS THE VERTICAL REINFORCEMENT. THE DOWEL SHALL EXTEND 48 BAR DIAMETERS VERTICALLY AND 20 BAR DIAMETERS INTO FOOTING

WHERE REINFORCING STEEL IS REQUIRED VERTICALLY DOWELS SHALL BE PROVIDED UNLESS NOTED OTHERWISE.

CONCRETE MASONRY UNITS (CMU) SHALL BE ERECTED AS LOAD BEARING CONCRETE MASONRY. COMPLY WITH ACI 530.1 "SPECIFICATION FOR MASONRY STRUCTURES" FOR MATERIALS, METHODS, AND WORKMANSHIP.

CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A NET COMPRESSIVE STRENGTH OF 1900 PSI.

CMU MINIMUM COMPRESSIVE STRENGTH (fm) SHALL BE 1500 PSI. MORTAR SHALL BE TYPE "M"OR "S" CONFORMING TO ASTM C270.

IN MASONRY SHALL BE A MINIMUM OF 48 BAR DIAMETERS.

ALL CELLS CONTAINING REINFORCEMENT, CELLS BELOW GRADE, AND ANY LOCATIONS NOTED ON THE PLANS SHALL BE GROUTED SOLID. GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH (fm) OF 2000 PSI.

PROVIDE PREFABRICATED " LADDER-WIRE "JOINT REINFORCEMENT AT 16" O.C. VERTICALLY. REINFORCEMENT SHALL HAVE AT LEAST ONE CROSS WIRE OF AT LEAST NO. 9 GAGE STEEL FOR EACH TWO SQUARE FEET OF WALL AREA. LONGITUDINAL WIRES SHALL BE THOROUGHLY EMBEDDED IN THE BED JOINT MORTAR. JOINT REINFORCEMENT SHALL BE INSTALLED IN THE FIRST TWO BED JOINTS ABOVE LINTELS AT OPENINGS. JOINT REINFORCEMENT SHALL NOT EXTEND THROUGH CONTROL JOINTS.

BEAMS OR GIRDERS (OR POINT LOADS TRANSMITTED TO THE CMU WALL BY A COLUMN) WHICH HAVE A REACTION EXCEEDING 10 KIPS (AS NOTED ON THE DRAWINGS) SUPPORTED ON CMU WALLS SHALL HAVE A MINIMUM BEARING LENGTH OF 8 INCHES OR THE FULL WIDTH OF THE CMU WALL WHICHEVER IS LESS. THE BLOCK UNDER THE BEAM SHALL BE GROUTED SOLID FOR A DISTANCE OF THE BEARING AREA PLUS FOUR TIMES THE WALL THICKNESS. WHERE VERTICAL REINFORCING STEEL IS REQUIRED IN THE CMU WALL THE BEARING AREA SHALL HAVE A MINIMUM OF TWO REINFORCING BARS CONTINUOUS TO THE FOOTING. THESE ADDITIONAL BARS SHALL MATCH THE VERTICAL BAR SIZE.

LOOSE STEEL ANGLES SERVING AS LINTELS FOR BRICK VENEER SHALL HAVE A MINIMUM OF 6" OF BEARING UNLESS NOTED OTHERWISE

PROVIDE A SOLID GROUT COURSE AT THE TOP OF THE MASONRY ELEVATIONS.

STRUCTURAL WOOD PANELS

FABRICATION, AND PLACEMENT OF STRUCTURAL SHEATHING SHALL BE IN ACCORDANCE WITH THE APA DESIGN / CONSTRUCTION GUIDE " RESIDENTIAL AND COMMERCIAL". AND ALL OTHER APPLICABLE APA STANDARDS.

ALL STRUCTURALLY REQUIRED SHEATHING SHALL BEAR THE MARK OF THE APA.

WALL SHEATHING SHALL BE APA RATED STRUCTURAL I SHEATHING. WALL SHEATHING SHALL BE ATTACHED TO ITS SUPPORTING WALL FRAMING WITH 1-8d CC NAIL AT 6" O.C. AT PANELS EDGES AND @ 12" O.C. IN PANEL FIELD UNLESS OTHERWISE NOTED ON THE PLANS. SHEATHING SHALL HAVE A SPAN RATING CONSTANT WITH THE FRAMING SPACING. APPLY BUILDING PAPER OVER THE SHEATHING AS

ROOF SHEATHING SHALL BE APA RATED SHEATHING EXPOSURE 1 OR 2. ROOF SHEATHING SHALL BE CONTINUOUS OVER TWO SUPPORTS AND ATTACHED TO ITS SUPPORTING ROOF FRAMING WITH 1-8d CC NAIL AT 6" O.C. AT PANELS EDGES AND @ 12" O.C. IN PANEL FIELD UNLESS OTHERWISE NOTED ON THE PLANS. SHEATHING SHALL BE APPLIED PERPENDICULAR TO FRAMING. SHEATHING SHALL HAVE A SPAN RATING CONSTANT WITH THE FRAMING SPACING. USE SUITABLE EDGE SUPPORT BY USE OF PLYWOOD CLIPS OR LUMBER BLOCKING UNLESS OTHERWISE NOTED. PANEL END JOINTS SHALL OCCUR OVER FRAMING. APPLY BUILDING PAPER OVER THE SHEATHING AS REQUIRED.

FLOOR SHEATHING SHALL BE APA RATED SHEATHING EXPOSURE 1 OR 2. ATTACH SHEATHING TO ITS SUPPORTING FRAMING WITH 1-8d CC NAIL AT 6" O.C. AT PANELS EDGES AND @ 12" O.C. IN PANEL FIELD UNLESS OTHERWISE NOTED ON THE PLANS. SHEATHING SHALL BE APPLIED PERPENDICULAR TO FRAMING. SHEATHING SHALL HAVE A SPAN RATING CONSTANT WITH THE FRAMING SPACING. USE SUITABLE EDGE SUPPORT BY USE OF T&G PLYWOOD OR LUMBER BLOCKING UNLESS OTHERWISE NOTED. PANEL END JOINTS SHALL OCCUR OVER FRAMING. APPLY BUILDING PAPER OVER THE SHEATHING AS REQUIRED.

SHEATHING SHALL HAVE A 1/8" GAP AT PANEL ENDS AND EDGES AS RECOMMENDED IN ACCORDANCE WITH THE APA.

SOLID SAWN WOOD FRAMING SHALL CONFORM TO THE SPECIFICATIONS AS LISTED IN THE NATIONAL FOREST PRODUCTS ASSOCIATION NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" LATEST EDITION (NDS). THE FRAMING SHALL BE OF THE SPECIES AND GRADE

1.1 JOISTS, RAFTERS, AND WOOD GIRDERS AND BEAMS SPRUCE PINE FIR No. 2 SPRUCE PINE FIR No. 3 OR STUD GRADE LVL OR PSL SHALL THE FOLLOWING MINIMUM DESIGN STRESSES:

THE SAME REQUIREMENT AS STUD WALLS.

 $E = 1.9 \times 10E6$ 2.2 Fb = 2600 PSI

2.3 Fv = 285 PSI2.4 Fc = 700 PSI

LUMBER IN CONTACT WITH CONCRETE, MASONRY, OR EARTH SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED TIMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2

NAILS SHALL BE COMMON WIRE NAILS UNLESS OTHERWISE NOTED.

LAG SCREWS SHALL CONFORM TO ANSI / ASME STANDARD B18.2.1-1981. LEAD HOLES FOR LAG SCREWS SHALL BE IN ACCORDANCE WITH NDS

ALL BEAM BEARING ON TIMBER FRAMING SHALL HAVE FULL BEARING FOR THE WIDTH OF THE BEAM AND SUPPORTED BY A MINIMUM OF THREE STUDS. WHERE BEAMS BEAR ONTO A WALL PARALLEL TO THE BEAM THE REAM SHALL HAVE A MINIMUM BEARING LENGTH OF 4-1/2"

STUDS WALLS SHALL CONSIST OF 2X4 STUDS @ 16" O.C. UNLESS OTHERWISE NOTED. STUDS SHALL BE CONTINUOUS FROM THE SOLE PLATE TO THE DBL. TOP PLATE AT THE CEILING OR ROOF. STUDS SHALL ONLY BE DISCONTINUOUS AT BEAMS / HEADERS FOR WINDOW OR DOOR

OPENINGS. HEADER SHALL HAVE A MINIMUM OF ONE KING STUD AT EACH END OF THE HEADER. KING STUDS SHALL BE CONTINUOUS WITH

INDIVIDUAL STUDS FORMING A COLUMN SHALL BE ATTACHED TOGETHER WITH ONE 10d CC NAIL @ 6" O C STAGGERDED THE STUD COLUMN SHALL BE CONTINUOUS TO THE FOUNDATION OR BEAM. THE COLUMN SHALL BE PROPERLY BLOCKED AT ALL FLOOR LEVELS TO ENSURE

BEAMS CONTAINING MULTIPLE PLIES OF LUMBER SHALL HAVE EACH PLY ATTACHED TO ITS ADJACENT PLY WITH 3 12d CC NAILS @ 12" O.C.

FLITCH PLATE BEAMS SHALL BE ATTACHED W/ 1/2" THROUGH BOLTS AT 24" O.C. STAGGERED W/ (2) BOLTS 6" FROM EA. END.

STEEL BEAMS SHALL BE ATTACHED TO EACH COLUMN SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDING THE JOISTS ARE TOE NAILED TO THE SOLE PLATE, AND THE SOLE PLATE IS NAILED OR BOLTED TO THE

STRUCTURAL STEEL

STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND VOLUMES I AND II OF THE MANUAL OF STEEL CONSTRUCTION "LOAD RESISTANCE FACTOR DESIGN" LATEST EDITION.

STRUCTURAL STEEL SHALL RECEIVE ONE COAT OF SHOP APPLIED RUST-INHIBITIVE PAINT. STEEL WHICH TO IS EMBEDDED IN CONCRETE, TOP FLANGES OF BEAMS WHICH HAVE SHEAR STUDS OR STEEL THAT WILL RECEIVE SPRAY-ON FIREPROOFING SHALL NOT BE PAINTED.

ALL STEEL SHALL HAVE THE FOLLOWING MINIMUM YIELD STRESS (Fy) LISTED BELOW:

3.1 STEEL WIDE FLANGE BEAMS AND COLUMNS STEEL PIPE COLUMNS TUBE STEEL 3.4 STEEL ANGLES AND MISC. SHAPES

WELDING SHALL CONFORM TO AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE AWS D1.1 LATEST EDITION. ELECTRODES FOR SHOP AND FIELD WELDING SHALL BE CLASS E70XX. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS PER THE ABOVE STANDARDS.

ANCHOR BOLTS SHALL BE A36 STEEL UNLESS OTHERWISE NOTED.

STEEL TO STEEL CONNECTIONS SHALL BE DESIGNED BY THE STEEL FABRICATOR FOR THE REACTIONS AS SHOWN ON THE DRAWINGS (UNFACTORED REACTIONS) USING THE STANDARD SECTIONS AND DETAILS AS SHOWN ON THESE DRAWINGS. HOWEVER ALL CONNECTIONS SHALL USE A MINIMUM OF 3/4" A-325 BOLTS AND WITH THE MINIMUM NUMBER OF BOLTS AS LISTED BELOW:

6.1 W8, W10, W12 BEAMS 2 ROWS OF BOLTS 6.2 W14, W16 BEAMS 3 ROWS OF BOLTS 6.3 W18 BEAMS 4 ROWS OF BOLTS 6.4 W21 BEAMS 5 ROWS OF BOLTS

BOLTS SHALL BE TIGHTENED TO A SNUG TYPE CONDITION UNO. THIS CONDITION IS ACHIEVED WHEN ALL PLIES OF A CONNECTION ARE IN FIRM CONTACT. BOLTS IN TENSION, COMBINED SHEAR AND TENSION OR REQUIRED BY LRFD SPECIFICATION SECTION J1.11 MUST BE FULLY TENSIONED IN BEARING TYPE CONNECTION.

ALL FILLET WELDS SHALL BE A MINIMUM OF ~4 INCH UNLESS NOTED OTHERWISE

EXTERIOR WOOD FRAMED DECKS

A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO THE STRUCTURE OR FREESTANDING. ROOFED

SUPPORT POSTS FOR DECKS SHALL BE SUPPORTED ON A CAST-IN-PLACE CONCRETE FOOTING. SEE CONCRETE SECTION OF THESE SPECIFICATIONS FOR CONCRETE REQUIREMENTS. THE SIZE OF THE FOOTING SHALL BE NOTED ON THE PLANS.

LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL REMAINING DECK LUMBER SHALL B PRESSURE TREATED IN ACCIDENCE WITH AWPA STANDARD C-2 OR A STANDARD GIVING EQUAL

WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A PRESSURE TREATED WOOD BAND FROM THE LENGTH OF THE DECK, OR METAL FLASHING SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AN THE STRUCTURE BAN SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT ON BRICK VENEER STRUCTURES AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY LASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AN THE DECK BAND. IF ATTACHED TO A BRICK VENEER STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT WITH THE BRICK

THE FOLLOWING SCHEDULE SHALL BE USED WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHMENT:

ALL STRUCTURES EXCEPT BRICK VENEER STRUCTURES

MAX. JOIST SPAN FASTENERS 5/8"Ø HDG BOLTS @ 42" O.C. <u>AND</u> 2-12d HDG NAILS @ 8" O.C. 5.2 16'-0" 5/8"Ø HDG BOLTS @ 20" O.C. <u>AND</u> 3-12d HDG NAILS @ 6" O.C. BRICK VENEER STRUCTURES

MAX. JOIST SPAN FASTENERS 5/8"Ø HDG BOLTS @ 24" O.C. 5.4 16'-0" 5/8"Ø HDG BOLTS @ 16" O.C. 5.5 NOTES: MIN. EDGE DISTANCE FOR BOLTS IS 2-1/2" MIN. NAIL PENETRATION IS 1-1/2"

THE FOLLOWING SCHEDULE SHALL BE USED FOR FLOOR DECKING. DECKING SHALL BE SYP No.2:

JOIST SPACING DECKING

A MIN. POST SIZE OF 6X6.

25 SF

8.4.3 8X8

6.2 16" O.C. 1-1/4" S4S (NOMINAL) OR 1" T&G (NOMINAL) 6.3 24" O.C. 2" S4S (NOMINAL)

SUPPORT POSTS SHALL BE SYP No.2. ALL POST HEIGHTS SHALL BE MEASURE FROM THE TOP OF THE FOOTING TO THE BOTTOM OF THE GIRDER.

POST SIZE MAX. TRIBUTARY AREA MAX. HEIGHT 25 SF 7.1 4X4 7.3 6X6 25 SF 14'-0" 7.3 8X8 20'-0"

DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING METHODS:

8.1 WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH 15.4 LATERAL BRACING IS NOT REQUIRED. WHEN THE BAND AND THE FLOOR JOISTS OF THE STRUCTURE ARE PARALLEL, FULL DEPTH BLOCKING SHALL BE INSTALLED @ 24" O.C. FOR A MIN. OF ONE JOIST SPACE ON THE STRUCTURE.

8.2 2X6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FO FREESTANDING DECKS OR PARALLEL O THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2X6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8" Ø HDG BOLT AT EACH BRACING MEMBER. BRACING SHALL RUN FROM THE GRADE LINE TO THE BOTTOM OF THE GIRDER.

8.3 4X4 WOOD KNEE BRACE MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE BRACE SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST. THE BRACES SHALL BE ANGLES BETWEEN 45° AND 60° FROM THE HORIZONTAL. ATTACH EACH BRACE TO THE POST AND THE GIRDER WITH 1-5/8" Ø HDG BOLT AT EACH END. THIS TYPE OF BRACING REQUIRES

POSTS MAY BE EMBEDDED IN 3000 PSI CONCRETE FOR LATERAL STABILITY AS PER THE FOLLOWING SCHEDULE:

POST SIZE MAX. TRIBUTARY MAX. HEIGHT EMBEDDED CONCRETE ABOVE TOP DEPTH DIAMETER OF FTG.

3'-6" 6'-0" 8.4.2 8X8 25 SF 10'-0" 4'-6" 2'-0" 12'-0" 5'-4" 2'-0"

WOOD TRUSSES

THE WOOD TRUSS MANUFACTURER / FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF THE WOOD TRUSSES. SUBMIT SEALED SHOP DRAWINGS AND SUPPORTING CALCULATION TO THE CONTRACTOR FOR REVIEW PRIOR TO FABRICATION. THE CONTRACTOR SHALL HAVE A MINIMUM OF FIVE (5) DAYS FOR REVIEW. THE REVIEW BY THE CONTRACTOR SHALL BE FOR OVERALL COMPLIANCE WITH THE DESIGN DOCUMENTS. THE CONTRACTOR SHALL ASSUME NO RESPONSIBILITY FOR THE CORRECTNESS FOR THE STRUCTURAL DESIGN FOR THE WOOD TRUSSES.

THE WOOD TRUSSES SHALL BE DESIGNED FOR ALL REQUIRED LOADINGS AS SPECIFIED IN THE NORTH CAROLINA RESIDENTIAL CODE, THE ASCE STANDARD "MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES" (ASCE 7-98). AND THE LOADING REQUIREMENT SHOWN ON THESE SPECIFICATIONS. THE TRUSS DRAWINGS SHALL BE COORDINATED WITH ALL OTHER CONSTRUCTION DOCUMENT AND PROVISIONS PROVIDED FOR LOADS SHOWN ON THESE DRAWINGS INCLUDING BUT NOT LIMITED TO HVAC EQUIPMENT, PIPING, AND ARCHITECTURAL FIXTURES ATTACHED TO THE TRUSSES.

THE TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD ${\tt CONSTRUCTION"} \ (\ {\tt NDS}\)\ {\tt LATEST}\ {\tt EDITION}\ {\tt AND}\ {\tt THE}\ {\tt LATEST}\ {\tt EDITION}\ {\tt OF}\ {\tt THE}\ "{\tt DESIGN}\ {\tt SPECIFICATION}\ {\tt FOR}\ {\tt METAL}\ {\tt PLATE}\ {\tt CONNECTED}\ {\tt WOOD}$

THE TRUSS MANUFACTURER SHALL PROVIDE ADEQUATE BRACING INFORMATION IN ACCORDANCE WITH "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING, AND BRACING METAL PLATE CONNECTED WOOD TRUSSES (HIB-91). THIS BRACING, BOTH TEMPORARY AND PERMANENT, SHALL BE SHOWN ON THE SHOP DRAWINGS. ALSO, THE SHOP DRAWINGS SHALL SHOW THE REQUIRED ATTACHMENTS FOR THE TRUSSES.

SUPPORT SHALL BE PROVIDED FOR ALL NON-LOAD BEARING PARTITIONS PARALLEL TO THE TRUSSES. THIS SUPPORT SHALL EITHER BE ACHIEVED BY INSTALLING AN EXTRA TRUSS UNDER THE PARTITION OR BY PROVIDING 2X BLOCKING ATTACHED TO EACH ADJACENT TRUSS WITH A JOIST HANGER. EITHER METHOD MAY BE USED, HOWEVER THIS DESIGN IS THE RESPONSIBLE OF THE TRUSS MANUFACTURER AND SHALL BE PROVIDED EVEN IF THE PARTITIONS ARE NOT SPECIFICALLY NOTED ON THE PROPOSED TRUSS LAYOUT.

ANY CHORDS OR TRUSS WEBS SHOWN ON THESE DRAWINGS HAVE BEEN SHOWN AS A REFERENCE ONLY. THE FINAL DESIGN OF THE TRUSSES SHALL BE PER THE MANUFACTURER.

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